

# PWT<sup>™</sup> Laminated Veneer Lumber and Rim Board PWT

**PR-L280** Revised May 6, 2024

Products: PWT<sup>™</sup> Laminated Veneer Lumber and Rim Board PWT, 1850 Park Lane, Burlington, Washington 98233 (888) 707-2285 www.pwtewp.com

- 1. Basis of the product report:
  - 2021, 2018, and 2015 International Building Code (IBC): Sections 104.11 Alternative materials and 2303.1.10 Structural composite lumber
  - 2012 IBC: Sections 104.11 Alternative materials and 2303.1.9 Structural composite lumber
  - 2021, 2018, and 2015 International Residential Code (IRC): Sections R104.11 Alternative materials, and R502.1.5, R602.1.5, and R802.1.4 Structural composite lumber
  - 2012 IRC: Sections R104.11 Alternative materials, and R502.1.7, R602.1.4, and R802.1.6 Structural composite lumber
  - ASTM D5456-18, ASTM D5456-14b, ASTM D5456-13, and ASTM D5456-09 recognized by the 2021 IBC and IRC, 2018 IBC and IRC, 2015 IBC and IRC, and 2012 IBC and IRC, respectively
  - ASTM D7672-14e1, ASTM D7672-14 and ASTM D7672-12 recognized by the 2021 IBC and IRC, 2018 IBC and IRC, and 2015 IBC and IRC, respectively
  - ANSI/APA PRR 410-2016 and ANSI/APA PRR 410-2011 recognized by the 2021 IBC and IRC and 2018 IBC and IRC, and 2015 IBC and IRC, respectively
  - 2021, 2015, and 2008 ANSI/AWC Special Design Provisions for Wind and Seismic (SDPWS) recognized by the 2021 and 2018, 2015, and 2012 IBC, respectively
  - PFS Corporation Test Reports: Gang-Lam LVL
  - APA Reports T2005P-76A, T2006P-06, T2008P-10, T2008P-31, T2008P-43, T2008P-113A, T2009P-12, T2010P-14, T2010P-18, T2011P-24, T2011P-30, T2011P-68, T2012P-08, T2012P-20, T2013P-15, T2014P-16, T2014P-26A, T2014P-27A, T2014P-28, T2014P-48, T2017P-09, T2017P-23, and T2018P-04, and other qualification data
- 2. Product description:

PWT<sup>™</sup> Laminated Veneer Lumber (LVL) is made with wood veneers laminated with grain parallel to the length of the member in accordance with the in-plant manufacturing standard approved by APA. PWT LVL is available with thicknesses up to 3-1/2 inches, and a range of widths and lengths. PWT LVL "Billet Beams" are fabricated by face-laminating primary thicknesses, and are available in thicknesses of 3-1/2, 5-1/4 or 7 inches. Refer to the manufacturer's technical guide (www.pwtewp.com) and a local PWT Wood Products distributor for product availability.

PWT LVL having a grade of 1.5E or greater can also be used as wall studs in conventional light-frame construction of the applicable code and in engineered wall systems. The minimum thickness of the PWT LVL for wall framing is 1-1/2 inches.

PWT LVL Rim Board is PWT LVL with two or more veneers oriented 90 degrees (cross-ply) to the length. PWT LVL Rim Board is available with a minimum thickness of 1-1/4 inches, and may be used for all applications applicable to PWT LVL except wall framing.

3. Design properties:

Table 1 lists the design properties, Table 2 lists the equivalent specific gravities for fastener design, Table 3 lists the allowable loads for rim boards, and Table 4 lists the allowable nail spacing for PWT LVL.

- 3.1 Beams, headers, and columns: The allowable loads for PWT LVL beams, headers, and columns shall be in accordance with the recommendations provided by the manufacturer (see link above).
- 3.2 Wall framing:
- 3.2.1 Prescriptive Stud Wall Applications: PWT LVL having a grade of 1.5E or greater used as studs in conventional construction are permitted in accordance with Sections 2308.5.9 of the 2021 and 2018 IBC, 2308.5.10 of the 2015 IBC, 2308.9.10 of the 2012 IBC, and R602.6 of the 2021 through 2012 IRC.
- 3.2.2 Engineered Stud Wall Applications: PWT LVL having a grade of 1.5E or greater shall be permitted in engineered wall applications when designed based on net section analysis in accordance with the National Design Specification for Wood Construction (NDS) and the restrictions specified in Section 4.3.2. The allowable design stress for bending, axial compression, and axial tension shall be multiplied by the strength reduction factors in Table 5 to account for stress concentrations in notches and holes.

The allowable shear values for nailed wood structural panel shear walls using PWT LVL having a grade of 1.5E or greater as the wall studs shall be determined using Table 4.3A of 2021, 2015, and 2008 SPDWS where the PWT LVL shall be considered to be equivalent to sawn lumber studs with a specific gravity of 0.50, when subjected to the nailing restrictions specified in Section 4.3.3 of this report.

- 4. Product installation:
- 4.1 Beams and headers:

PWT LVL shall be installed in accordance with the recommendations provided by the manufacturer (see link above). Permissible details and allowable hole sizes shall be in accordance with the recommendations provided by the manufacturer.

- 4.2 Columns:
- 4.2.1 PWT LVL used as free-standing columns shall not be drilled or notched without the approval of a professional engineer or the manufacturer. Bolts, lag screws, and self-tapping screws shall only be inserted through the face of the column, perpendicular to the face of the veneers in PWT LVL.
- 4.2.2 Built-up columns: When used for built-up columns, PWT LVL shall be constructed using connections specified by the manufacturer (see link above).
- 4.3 Wall framing:
- 4.3.1 Prescriptive stud wall applications: Cutting, notching, and boring of PWT LVL having a grade of 1.5E or greater used as studs in conventional construction are permitted in accordance with Sections 2308.5.9 of the 2021 and 2018 IBC, 2308.5.10 of the 2015 IBC, 2308.9.10 of the 2012 IBC, and R602.6 of the 2021 through 2012 IRC. Stud wall nailing restrictions and requirements are presented in Section 4.3.3 of this report.
- 4.3.2 Engineered stud wall applications: Cutting, notching, and boring of PWT LVL having a grade of 1.5E or greater shall be permitted in engineered wall applications with the following restrictions:

- a) Holes up to 40% of the stud depth are allowed anywhere in the stud height for bearing walls, except that a hole shall not be placed within 6 inches of either end of the stud. A minimum edge distance of 5/8 inch shall be maintained for all holes for stud depths of 5-1/2 inches (i.e., nominal 2x6) or less. For larger depths, a minimum edge distance of 12% of the stud depth shall be maintained for all holes.
- b) Notches up to 25% of the stud depth are allowed anywhere in the stud height, except that a notch shall not be placed within 6 inches of either end of the stud. The notch length shall not exceed 8 inches.
- c) Holes and notches shall not be cut at the same cross section, and the minimum clear vertical space between hole and notch shall be 2 times the hole diameter or 2 times the notch length, whichever is greater.
- d) Stud wall nailing restrictions and requirements are presented in Section 4.3.3.
- 4.3.3 Nailing restrictions and requirements
  - a) Attach wall plate to studs in accordance with the code.
  - b) Sheathing to 1.5E or greater PWT LVL Studs
    - For sheathing attached with 8d common nails (0.131 inch x 2-1/2 inches) or smaller with a spacing no closer than 6 inches on center, a single PWT LVL stud shall be permitted for framing at adjoining panel edges. Nails shall be installed a minimum 3/8 inch from all panel edges. 10d common nails (0.148 inch x 3 inches) are not allowed where a single PWT LVL stud is used at adjoining panel edges.
    - For sheathing attached with 10d common nails (0.148 inch x 3 inches) spaced no closer than 4 inches on center or 8d common nails (0.131 inch x 2-1/2 inches) spaced no closer than 3 inches on center a double, stitch-nailed, PWT LVL stud or single 2-1/2 inch thick PWT LVL stud is required at adjoining panel edges. Nails shall be installed a minimum 1/2 inch from all panel edges and shall be staggered a minimum of 1/4 inch for each row of nails.
  - c) Nails for attaching sheathing to studs shall not be spaced closer than 3 inches on center.
  - d) Maximum nail size for attaching sheathing to studs is 10d common (0.148 inch x 3 inches).
  - e) Stitch nailing for double studs
    - For stud wall applications in accordance with the IRC and the conventional lightframe construction provisions of the IBC, double PWT LVL studs shall be stitchnailed together with 2 staggered rows of nails (minimum 0.120 inch x 2-7/8 inches) spaced 8 inches in each row.
    - For engineered stud wall applications, the stitch nailing of double PWT LVL studs shall be designed to transfer the required lateral shear using an assumed equivalent specific gravity of 0.50.
- 4.4 Rim board:

PWT LVL rim boards shall be installed in accordance with the recommendations provided by the manufacturer (see link above) and the code.

- 5. Fire-rated assemblies:
- 5.1 Design of fire-resistant exposed wood members in accordance with Chapter 16 of the NDS, Section 722.1 of the 2021, 2018, and 2015 IBC, or Section 722.6.3 of the 2012 IBC shall be applicable to PWT LVL. Fire-rated assemblies shall be constructed in accordance with the recommendations provided by APA *Fire-Rated Systems*, Form W305 (www.apawood.org/resource-library), and the manufacturer.

- 5.2 When used as joists/rafters, PWT LVL having a grade of 1.5E or greater are permitted to be used as direct replacement for solid-sawn lumber having the same dimensions, in any fire-resistance-rated floor/roof assemblies listed in Table 721.1(3) of the 2021 through 2012 IBC.
- 5.3 The provisions of Section R302.13, Exception 4 of the 2021, 2018, and 2015 IRC and Section R501.3, Exception 4 of the 2012 IRC for fire protection of floors shall be applicable to floor assemblies constructed with PWT LVL having a grade of 1.5E or greater with a nominal 2x10 dimension (i.e., 1-1/2 inches by 9-1/4 inches net dimension) or greater.
- 5.4 When used as wall studs, PWT LVL having a grade of 1.5E or greater are permitted to be used as a direct replacement for solid-sawn lumber, having the same dimensions, in any fire-resistance-rated wall assemblies listed in Table 721.1(2) of the 2021 through 2012 IBC.
- 5.5 As an alternative to the prescriptive fire-resistance-rated wall assemblies listed in Table 721.1(2) of the 2021 through 2012 IBC, a one-hour fire-resistance-rated wall assembly shall be permitted to be designed and constructed with the limitations listed below, provided that the applied axial stress on each stud does not exceed 550 psi for 1.5E or greater PWT LVL. When the slenderness ratio exceeds 33, the Fc', which is the FcII value tabulated in Table 1 for each stud grade adjusted for all applicable adjustment factors, including column stability factor, in accordance with NDS, shall be multiplied by 0.63 for PWT LVL having a grade of 1.5E or greater.
  - a) The stud spacing shall be no greater than 24 inches on center,
  - b) The top and bottom plates of the wall shall be constructed in accordance with the nailing schedule specified in Table 2304.10.2 of the 2021 IBC, Table 2304.10.1 of the 2018 and 2015 IBC, or Table 2304.9.1 of the 2012 IBC or Table R602.3(1) of the 2021 through 2012 IRC.
  - c) The wall shall be covered with one layer of 5/8-inch Type X gypsum wall board attached to studs with 2-1/4-inch long Type S drywall screws at 7 inches on center on the perimeter and in the field, and
  - d) A minimum of 2.5 lbf/ft<sup>3</sup> mineral wool insulation shall be placed in the stud cavity.
- 6. Limitations:
  - a) PWT LVL shall be designed in accordance with the code using the design properties and installation requirements specified in this report.
  - b) PWT LVL is limited to dry service conditions where the equivalent moisture content of sawn lumber is less than 16%.
  - c) PWT LVL is produced at the PWT facilities in Wilmington, North Carolina, and Golden, British Columbia, Canada under a quality assurance program audited by APA. A list of the PWT LVL grades manufactured at different PWT facilities is maintained by APA for independent auditing purposes.
  - d) This report is subject to re-examination in one year.
- 7. Identification:

The PWT LVL described in this report are identified by a label bearing the manufacturer's name (PWT) and/or trademark, the APA assigned plant number (1071 for the Wilmington plant, and 1066 for the Golden plant), the product type and grade, the APA logo, the report number PR-L280, and a means of identifying the date of manufacture.

# Table 1. Design Properties (Allowable Stress Design) for PWT LVL<sup>(a,b)</sup>

Property		PWT LVL Rim Board (cross-ply)				PWT LVL			
		1550F <sub>b</sub> -1.3E	2250F <sub>b</sub> -1.5E	2250F <sub>b</sub> -1.55E	2650F <sub>b</sub> -1.9E	2900F <sub>b</sub> -2.0E	2950F <sub>b</sub> -2.0E	3100F <sub>b</sub> -2.1E	3100F <sub>b</sub> -2.2E
	Joist	1,550 <sup>(d)</sup>	2,250 <sup>(e)</sup>	2,250 <sup>(e)</sup>	2,650 <sup>(e)</sup>	2,900 <sup>(e)</sup>	2,950 <sup>(e)</sup>	3,100 <sup>(e)</sup>	3,100 <sup>(e)</sup>
Bending (F <sub>b</sub> ) <sup>(c)</sup> , psi	Plank	1,550	2,200 <sup>(f)</sup>	2,250 <sup>(f)</sup>	2,600 <sup>(f)</sup>	2,950 <sup>(f)</sup>	2,950 <sup>(f)</sup>	3,100 <sup>(f)</sup>	2,950 <sup>(f)</sup>
Tension parallel to grain (F <sub>t</sub> ),	psi	1,200 <sup>(g)</sup>	1,350 <sup>(g)</sup>	1,500 <sup>(g)</sup>	1,600 <sup>(g)</sup>	1,800 <sup>(g)</sup>	1,800 <sup>(g)</sup>	1,800 <sup>(g)</sup>	1,800 <sup>(g)</sup>
Longitudinal aboat (E) pai	Joist	250	285	255	285	285	290	290	290
Longitudinal shear (F <sub>v</sub> ), psi	Plank	140	140	150	140	140	140	140	140
Compression parallel (F <sub>c//</sub> ), ps	si	1,700	2,350	2,350	2,350	3,200	3,200	3,200	3,200
Compression perpendicular	Joist	680	750	750	750	750	750	750	750
(F <sub>c⊥</sub> ) <sup>(j)</sup> , psi	Plank <sup>(k)</sup>	550	550	650	550	550	550	550	550
Modulus of Elasticity (E),	Joist	1.30 <sup>(h)</sup>	1.50 <sup>(h)</sup>	1.55 <sup>(h)</sup>	1.90 <sup>(i)</sup>	2.00 <sup>(h)</sup>	2.00 <sup>(i)</sup>	2.10 <sup>(h)</sup>	2.20 <sup>(h)</sup>
10 <sup>6</sup> psi	Plank	1.10 <sup>(h)</sup>	1.40 <sup>(h)</sup>	1.55 <sup>(h)</sup>	1.80 <sup>(i)</sup>	2.00 <sup>(h)</sup>	2.00 <sup>(i)</sup>	2.00 <sup>(h)</sup>	2.20 <sup>(h)</sup>

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 lbf = 4.448 N, 1 psi = 6.9 kPa.

(a) The tabulated values are design values for normal duration of load. All values, except for E and F<sub>cL</sub>, are permitted to be adjusted for other load durations as permitted by the code. The design stresses are limited to conditions in which the equivalent moisture content of sawn lumber does not exceed 16%.

(b) Allowable stresses for "Joist" refer to loads applied parallel to the wide face of the strands (the edge of the member). "Plank" refers to loads applied perpendicular to the wide face of the strands (the face of the member).

<sup>(c)</sup> Tabulated flexural stress (F<sub>b</sub>) may be increased by 4% when the member qualifies as a repetitive member as defined in the NDS.

<sup>(d)</sup> The tabulated value for PWT LVL Rim Board (with cross plies) is based on a reference depth of 12 inches. For other depths, when loaded edgewise, the allowable bending stress (F<sub>b</sub>) shall be modified as follows, based on thickness:

- For thickness of 1 inch and 1-1/8 inches, multiply  $F_b$  by  $(12/d)^{0.323}$ .
- For thickness of 1-1/4 inches and greater, multiply F<sub>b</sub> by (12/d)<sup>0.261</sup>.

For depths less than 3-1/2 inches, the factor for the 3-1/2-inch depth shall be used.

(e) The tabulated values for PWT LVL are based on a reference depth of 12 inches. For depths greater than 12 inches, multiply F<sub>b</sub> by (12/d)<sup>0.143</sup>. For depths less than 12 inches, multiply F<sub>b</sub> by (12/d)<sup>0.111</sup>. For depths less than 3-1/2 inches, the factor for the 3-1/2-inch depth shall be used.

<sup>(f)</sup> For PWT LVL "Billet Beams" up to 7 inches in thickness (see Section 2.2), the allowable bending stress (F<sub>b</sub>) in plank orientation shall be modified by (1.75/d)<sup>0.25</sup> ≤ 1.0.

(9) The values for PWT LVL published in Table 1 are based on a reference length of 3 feet. For other lengths, the allowable tensile stress shall be modified by (3/L)<sup>0.111</sup>, where L = length in feet. For lengths less than 3 feet, use the allowable tension stresses in Table 1.

<sup>(h)</sup> The tabulated modulus of elasticity is the shear-free MOE. For uniformly loaded simple-span beams deflection is calculated as follows:

$$=\frac{270wL^4}{Ebh^3} + \frac{28.8wL^2}{Ebh}$$
 [Eq. 1]

Where:	δ	=	estimated deflection, inches,	w	=	uniform load, plf
	L	=	span, feet,	E	=	tabulated modulus of elasticity, psi
	В	=	beam width, inches, and	h	=	beam depth, inches

δ

(Footnotes continue on next page)

- (i) The tabulated modulus of elasticity is the apparent MOE, which includes the effects of shear deformation. When calculating deflection, standard engineering formulae for pure bending deflection are sufficient, and the second term of Equation 1 in Footnote h may be ignored.
- <sup>(I)</sup> When designing with the tabulated compressive stress perpendicular to grain (F<sub>c1</sub>), the Bearing Area Factor (C<sub>b</sub>) stipulated in Section 3.10.4 of the NDS shall be permitted to be applied.
- (k) The tabulated compressive stress perpendicular to grain (F<sub>c1</sub>) value is based on the average stress at the proportional limit or 0.04-in. deformation, whichever is less, in accordance with ASTM D5456.

# Table 2. Fastener Design for PWT LVL<sup>(a,b,c)</sup>

		Equivalent Spec	ific Gravity (S.G.)				
Na	ails	Nails and W	lood Screws	Bolts and Lag Screws <sup>(d,e)</sup>			
Withdray	wal Load	Latera	I Load	Lateral Load			
				Installed in Face			
Installed in Edge	Installed in Face	Installed in Edge	Installed in Face	Parallel to Grain	Perpendicular to Grain		
PWT LVL Rim Board (cross-ply)							
0.46	0.50	0.50	0.50	0.46	0.50		
		PWT	LVL				
0.46	0.50	0.50	0.50	0.46	0.50		

<sup>(a)</sup> Fastener types and orientation not specifically described above are beyond the scope of this report.

(b) Fastener design values calculated using the tabulated equivalent specific gravities given above must be adjusted by the applicable adjustment factors specified in the NDS for connection.

(c) Fastener spacing, and end and edge distances must be as specified in the NDS, except that nail spacing and end distance must be as specified in Table 4.

<sup>(d)</sup> Bolts and lag screws shall only be installed into the face (plank orientation) of the PWT LVL.

The capacities for 1/2-in. (12.7-mm) diameter lag screws installed into PWT LVL Rim Board for ledge attachment shall be in accordance with Table 3.

Table 3.	Allowabl	e Loads for	PWT	LVL Rim	n Boards <sup>(a)</sup>

Grade	Thickness (in.)	Lateral Load <sup>(b,c)</sup>	Vertical Uniform Load <sup>(d)</sup> (lbf/ft)		Vertical Concentrated Load (lbf)	Lateral Resistance for 1/2- inch-dia.	
		(lbf/ft)	Depth ≤ 16"	16" < Depth ≤ 24"	Depth ≤ 24"	Lag Screws (lbf)	
PWT LVL Rim Board (cross-ply)							
1550F <sub>b</sub> -1.3E	≥ 1-1/4	250	9,350	5,070	4,210	450	
PWT LVL							
1 FE and above	1-1/2 ≤ t <1-3/4	250	4,000	2,500	2,700	450	
1.5E and above	t ≥ 1-3/4	250	4,500	3,450	3,200	450	

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 lbf = 4.448 N, 1 lbf/ft = 14.6 N/m.

(a) The tabulated design values are applicable to the normal load duration (10 years) for wood products, except for the lateral load capacity, which is based on the short-term load duration (10 minutes). Design values shall be adjusted for other load durations in accordance with the applicable building code except that the vertical uniform load capacity and vertical concentrated load capacity are not permitted to be increased for any load durations shorter than the normal load duration (10 years).

(b) Toe-nailed connections are not limited by the 150 lbf/ft lateral load capacity noted for Seismic Design Categories D, E and F in Section 4.1.10 of the 2021 SDPWS and Section 4.1.7 of the 2015 and 2008 SDPWS.

The nailing schedule for sheathing to rim and rim board to sill plate (toe-nailed) is based on 8d box (0.113 inch x 2-1/2 (c) inches) nails at 6 inches on center (refer to APA W345, APA Performance Rated Rim Boards® for full details). Lateral load capacity is permitted to be increased by a factor of 1.4 when subjected to wind loads. Commercial framing connectors may be used to achieve lateral load capacities exceeding the values shown in this table. Calculations shall be based on the equivalent specific gravity values listed in Table 2 subjected to the nailing spacing provided in Table 4.

The allowable vertical uniform load capacity is based on the strength of the rim board and may need to be reduced based on the bearing capacity of the supporting wall plate.

Thickness (in.)	Orientation <sup>(e)</sup>	Common Nail	Minimum End	Minimum Nail S	pacing per Row (in.)
THICKNESS (III.)	Onentation	Size <sup>(f,g)</sup>	Distance (in.)	Single Row	Multiple Rows (c,d)
		8d & smaller	2-1/2	4	
	Edge <sup>(h)</sup>	10d & 12d	2-1/2	4	NA
< 1-1/2"		16d	3-1/2	5	
< 1-1/2		8d & smaller	1-1/2	3	3
	Face(i)	10d & 12d	1-1/2	3	3
		16d	1-1/2	5	5
	Edge <sup>(h)</sup>	8d & smaller	2-1/2	3	3
		10d & 12d	2-1/2	4	4
≥ 1-1/2"		16d	3-1/2	5	5
≥ 1-1/2		8d & smaller	1-1/2	3	3
	Face <sup>(i)</sup>	10d & 12d	1-1/2	3	3
		16d	1-1/2	5	5

### Table 4. Minimum Allowable Nail Spacings for PWT LVL<sup>(a,b)</sup>

For SI: 1 inch = 25.4 mm.

<sup>(a)</sup> Spacing requirements and maximum nail size for panel edge nailing of wall sheathing at adjoining panels must be in accordance with Section 4.3.3.

- <sup>(b)</sup> Edge distance shall be sufficient to prevent splitting.
- <sup>(6)</sup> Multiple rows must be spaced 1/2 inch or more from each other and offset one-half of the tabulated minimum nail spacing, as shown in Figure 1.
- <sup>(d)</sup> Multiple rows must be equally spaced about the centerline of the edge or face (whichever applies).
- (e) Face orientation applies to nails driven into the face of the PWT LVL member, such that the long axis of the nail is perpendicular to the wide faces of the strands or veneers. Edge orientation applies to nails driven into the edge of the PWT LVL member.
- <sup>(f)</sup> 16d sinkers (0.148 inch x 3-1/4 inches) may be spaced the same as a 12d common wire nail (0.148 inch x 3-1/4 inches).
- <sup>(9)</sup> Nails listed are common wire nails. For box nails, the spacing and end distance requirements of the next shorter common nail may be used: e.g., a 16d box nail may be spaced the same as a 10d and 12d common nail. Larger nail sizes and shank types not specifically described above are beyond the scope of this report.
- (h) Nail penetration for edge nailing shall not exceed 2 inches for 16d common nails (0.162 inch x 3-1/2 inches) and 2-1/2 inches for all nails with a smaller shank diameter.
- <sup>(i)</sup> Tabulated closest on-center spacing for face orientation is applicable to nails that are installed in rows parallel to the grain (length) of the PWT LVL. For nails installed in rows perpendicular to the direction of grain (width/depth) of the PWT LVL, the closest on-center spacing for face orientation shall be sufficient to prevent splitting of the PWT LVL.

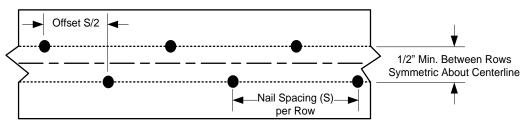


Figure 1. Spacing of multiple rows of nails.

Table 5. Strength Reduction Factors for Notches and Holes in PWT LVL Studs <sup>(a,t)</sup>	Table 5.	Strength Reduction	Factors for Notches	and Holes in F	PWT LVL Studs <sup>(a,b,</sup>
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Motorial		Notches		Holes			
Material	Bending	Compression	Tension	Bending	Compression	Tension	
PWT LVL	0.80	0.90	0.60	0.95	0.95	0.95	

(a) Design of PWT LVL studs with notches and holes used in engineered wall framing must be based on a net-section analysis in accordance with the NDS. See Section 4.3.2 of this report for limitations on the allowed size and placement of notches and holes.

(b) The reference design stresses for bending, compression and tension from Table 1 must be multiplied by the strength reduction factors in the above table.

<sup>(c)</sup> See Section 4.3.1 for notching and boring of holes in PWT LVL studs used in prescriptive wall framing.

APA – The Engineered Wood Association is an approved national standards developer accredited by American National Standards Institute (ANSI). APA publishes ANSI standards and Voluntary Product Standards for wood structural panels and engineered wood products. APA is an accredited certification body under ISO/IEC 17065 by Standards Council of Canada (SCC), an accredited inspection agency under ISO/IEC 17020 by International Code Council (ICC) International Accreditation Service (IAS), and an accredited testing organization under ISO/IEC 17025 by IAS. APA is also an approved Product Certification Agency, Testing Laboratory, Quality Assurance Entity, Validation Entity, and Product Evaluation Entity by the State of Florida, and an approved testing laboratory by City of Los Angeles.

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