Mississippi Laminators Glulam Products
Mississippi Laminators, Inc.

Products: Mississippi Laminators Glulam Products
Mississippi Laminators, Inc., 1151 County Road 210, P.O. Box 405, Shubuta, Mississippi, 39360
(601) 687-1571

1. Basis of the product report:
     Alternative materials and 2303.1.3 Structural glued laminated timber
   • 2021, 2018, and 2015 International Residential Code (IRC): Sections R104.11 Alternative
     materials, and R502.1.3, R602.1.3, and R802.1.2 Structural glued laminated timber
   • 2012 IRC: Sections R104.11 Alternative materials, and R502.1.5, R602.1.2, and
     R802.1.4 Structural glued laminated timber
   • ANSI 117-2020 and ANSI 117-2015 recognized in the 2021 IBC and IRC, and 2018 IBC
     and IRC, respectively
     2021 and 2018 IBC and IRC, 2015 IBC and IRC, and 2012 IBC and IRC, respectively
   • ASTM D3737-18e1, D3737-12, and D3737-08 recognized in the 2021 IBC and IRC, 2018
     and 2015 IBC and IRC, and 2012 IBC and IRC, respectively
   • Qualification data

2. Product description:
   Mississippi Laminators glulam products are structural glued laminated timber manufactured
   with Southern Pine laminations in accordance with ANSI A190.1 using the layup
   combinations recognized in the National Design Specification (NDS) Supplement and/or
   ANSI 117.

   The adhesives used to manufacture the glulam products are exterior-type adhesives
   meeting the requirements of ASTM D2559. Mississippi Laminators glulam products are
   used as beams, headers, rafters, or purlins, and are manufactured in nominal widths of 4, 6,
   and 8 inches, depths to 39 inches, and lengths up to 52 feet.

3. Design properties:
   Tables 1 and 2 list the allowable design properties for Mississippi Laminators glulam
   products. The allowable spans for Mississippi Laminators glulam products shall be in
   accordance with APA Data File: Glued Laminated Beam Design Tables, Form S475
   (www.apawood.org/resource-library), or shall be determined based on the design properties
   listed in Tables 1 or 2, as appropriate.

4. Product installation:
   Mississippi Laminators glulam products shall be installed in accordance with the
   recommendations provided by the manufacturer and APA Construction Guide: Glulam
   Connection Details, Form T300 (see link above). Permissible field notching and drilling shall
   be in accordance with the recommendations provided by the manufacturer and APA
   Technical Notes: Field Notching and Drilling of Glued Laminated Timber Beams, Form
   S560, and Effect of Large Diameter Horizontal Holes on the Bending and Shear Properties of
   Structural Glued Laminated Timber, Form V700 (see link above).

5. Fire-rated assemblies:
   Design of fire-resistant exposed wood members in accordance with Chapter 16 of the
   National Design Specification for Wood Construction (NDS), Section 722.1 of the 2021,
2018, and 2015 IBC, or Section 722.6.3 of the 2012 IBC shall be applicable to Mississippi Laminators glulam products. Fire-rated assemblies shall be constructed in accordance with the recommendations provided by the manufacturer, and APA Design/Construction Guide: *Fire-Rated Systems*, Form W305 (see link above).

6. Limitations:
   a) Mississippi Laminators glulam products shall be designed in accordance with the code using the design properties specified in this report.
   b) Mississippi Laminators glulam products are produced at the Shubuta, Mississippi facility under a quality assurance program audited by APA.
   c) This report is subject to re-examination in one year.

7. Identification:
Mississippi Laminators glulam products described in this report are identified by a label bearing the manufacturer's name (Mississippi Laminators, Inc.) and/or trademark, the APA assigned plant number (1074), the product standard (ANSI A190.1), the APA logo, the layup combination symbol, the report number PR-L297, and a means of identifying the date of manufacture.
Table 1. Allowable Design Values for Glulam Products Manufactured by Mississippi Laminators, Inc. Stressed Primarily in Bending for Normal Duration of Load

<table>
<thead>
<tr>
<th>Symbol</th>
<th>Species</th>
<th>Outer/ Core (B)</th>
<th>Compression Perpendicular to Grain</th>
<th>Extreme Fiber in Bending (Positive Bending)</th>
<th>Bottom of Beam Stressed in Tension</th>
<th>Top of Beam Stressed in Tension (Negative Bending)</th>
<th>Shear Parallel to Grain (Bending)</th>
<th>True E (true) (psi)</th>
<th>Apparent E (app) (psi)</th>
<th>Beam Stability</th>
<th>Extreme Fiber in Bending (Grain)</th>
<th>Modulus of Elasticity (psi)</th>
<th>Comp. Perpendicular to Grain</th>
<th>Shear Parallel to Grain (Grain)</th>
<th>Modulus of Elasticity (psi)</th>
<th>Axially Loaded</th>
<th>Fasteners</th>
</tr>
</thead>
<tbody>
<tr>
<td>24F-V1</td>
<td>SP/SP (U)</td>
<td>2,400</td>
<td>1,750</td>
<td>740</td>
<td>650</td>
<td>300</td>
<td>1.8</td>
<td>1.7</td>
<td>0.90</td>
<td>1,450</td>
<td>650</td>
<td>260</td>
<td>1.5</td>
<td>1.5</td>
<td>0.79</td>
<td>1,100</td>
<td>1,500</td>
</tr>
<tr>
<td>24F-V3</td>
<td>SP/SP (U)</td>
<td>2,400</td>
<td>2,000</td>
<td>740</td>
<td>740</td>
<td>300</td>
<td>1.9</td>
<td>1.8</td>
<td>0.95</td>
<td>1,700</td>
<td>650</td>
<td>260</td>
<td>1.7</td>
<td>1.6</td>
<td>0.85</td>
<td>1,150</td>
<td>1,650</td>
</tr>
<tr>
<td>24F-V4(1)</td>
<td>SP/SP (U)</td>
<td>2,400</td>
<td>1,650</td>
<td>740</td>
<td>650</td>
<td>210</td>
<td>1.8</td>
<td>1.7</td>
<td>0.90</td>
<td>1,350</td>
<td>470</td>
<td>230</td>
<td>1.6</td>
<td>1.5</td>
<td>0.79</td>
<td>975</td>
<td>1,350</td>
</tr>
<tr>
<td>24F-V5</td>
<td>SP/SP (B)</td>
<td>2,400</td>
<td>2,400</td>
<td>740</td>
<td>740</td>
<td>300</td>
<td>1.8</td>
<td>1.7</td>
<td>0.90</td>
<td>1,700</td>
<td>650</td>
<td>260</td>
<td>1.7</td>
<td>1.6</td>
<td>0.85</td>
<td>1,150</td>
<td>1,600</td>
</tr>
<tr>
<td>24F-V5M1</td>
<td>SP/SP (B)</td>
<td>2,400</td>
<td>2,400</td>
<td>740</td>
<td>740</td>
<td>300</td>
<td>1.9</td>
<td>1.8</td>
<td>0.95</td>
<td>1,700</td>
<td>650</td>
<td>260</td>
<td>1.7</td>
<td>1.6</td>
<td>0.85</td>
<td>1,150</td>
<td>1,600</td>
</tr>
<tr>
<td>26F-V1</td>
<td>SP/SP (U)</td>
<td>2,600</td>
<td>2,000</td>
<td>740</td>
<td>740</td>
<td>300</td>
<td>1.9</td>
<td>1.8</td>
<td>0.95</td>
<td>1,700</td>
<td>650</td>
<td>260</td>
<td>1.7</td>
<td>1.6</td>
<td>0.85</td>
<td>1,150</td>
<td>1,600</td>
</tr>
<tr>
<td>26F-V3</td>
<td>SP/SP (U)</td>
<td>2,600</td>
<td>2,100</td>
<td>740</td>
<td>740</td>
<td>300</td>
<td>2.0</td>
<td>1.9</td>
<td>1.00</td>
<td>1,950</td>
<td>650</td>
<td>260</td>
<td>1.9</td>
<td>1.8</td>
<td>0.95</td>
<td>1,250</td>
<td>1,800</td>
</tr>
<tr>
<td>26F-V3M1(1)</td>
<td>SP/SP (U)</td>
<td>2,600</td>
<td>2,100</td>
<td>740</td>
<td>740</td>
<td>300</td>
<td>2.0</td>
<td>1.9</td>
<td>1.00</td>
<td>1,950</td>
<td>650</td>
<td>260</td>
<td>1.9</td>
<td>1.8</td>
<td>0.95</td>
<td>1,250</td>
<td>1,800</td>
</tr>
<tr>
<td>26F-V4</td>
<td>SP/SP (B)</td>
<td>2,600</td>
<td>2,600</td>
<td>740</td>
<td>740</td>
<td>300</td>
<td>2.0</td>
<td>1.9</td>
<td>1.00</td>
<td>1,700</td>
<td>650</td>
<td>260</td>
<td>1.9</td>
<td>1.8</td>
<td>0.95</td>
<td>1,200</td>
<td>1,600</td>
</tr>
<tr>
<td>26F-V4M1(1)</td>
<td>SP/SP (B)</td>
<td>2,600</td>
<td>2,600</td>
<td>740</td>
<td>740</td>
<td>300</td>
<td>2.0</td>
<td>1.9</td>
<td>1.00</td>
<td>1,700</td>
<td>650</td>
<td>260</td>
<td>1.9</td>
<td>1.8</td>
<td>0.95</td>
<td>1,200</td>
<td>1,600</td>
</tr>
<tr>
<td>28F-E1</td>
<td>SP/SP (U)</td>
<td>2,800</td>
<td>2,300</td>
<td>805</td>
<td>805</td>
<td>300</td>
<td>2.2(1)</td>
<td>2.1(1)</td>
<td>1.11(1)</td>
<td>1,600</td>
<td>650</td>
<td>260</td>
<td>1.8</td>
<td>1.7</td>
<td>0.90</td>
<td>1,300</td>
<td>1,850</td>
</tr>
<tr>
<td>28F-E2</td>
<td>SP/SP (B)</td>
<td>2,800</td>
<td>2,800</td>
<td>805</td>
<td>805</td>
<td>300</td>
<td>2.2(1)</td>
<td>2.1(1)</td>
<td>1.11(1)</td>
<td>2,000</td>
<td>650</td>
<td>260</td>
<td>1.8</td>
<td>1.7</td>
<td>0.90</td>
<td>1,300</td>
<td>1,850</td>
</tr>
<tr>
<td>30F-E1(1)</td>
<td>SP/SP (B)</td>
<td>3,000</td>
<td>2,400</td>
<td>805</td>
<td>805</td>
<td>300</td>
<td>2.2(1)</td>
<td>2.1(1)</td>
<td>1.11(1)</td>
<td>1,750</td>
<td>650</td>
<td>260</td>
<td>1.8</td>
<td>1.7</td>
<td>0.90</td>
<td>1,250</td>
<td>1,750</td>
</tr>
<tr>
<td>30F-E2(1)</td>
<td>SP/SP (B)</td>
<td>3,000</td>
<td>3,000</td>
<td>805</td>
<td>805</td>
<td>300</td>
<td>2.2(1)</td>
<td>2.1(1)</td>
<td>1.11(1)</td>
<td>1,750</td>
<td>650</td>
<td>260</td>
<td>1.8</td>
<td>1.7</td>
<td>0.90</td>
<td>1,350</td>
<td>1,750</td>
</tr>
</tbody>
</table>

| Use-wet factor | 0.8 | 0.53 | 0.879 | 0.833 | 0.8 | 0.53 | 0.875 | 0.833 | 0.8 | 0.73 | see NDS |

(1) The combinations in this table are intended primarily for members stressed in bending due to loads applied perpendicular to the wide faces of the laminations. Allowable design values are tabulated, however, for loading both perpendicular and parallel to the wide faces of the laminations.

(2) The tabulated allowable design values are for normal duration of loading. For other durations of loading, see the applicable building code. The tabulated allowable design values are for dry conditions of use. For wet conditions of use, multiply the tabulated values by the wet-use factors shown at the bottom of the table.

(3) SP = Southern pine.

(4) The unbalanced (U) layout is intended primarily for simple-span applications and the balanced (B) layout is intended primarily for continuous or cantilevered applications.

(5) The values of E are based on members 5-1/8 inches in width by 12 inches in depth by 21 feet in length. For members with a larger volume, F_SG shall be multiplied by a volume factor, C_SG = (5.125/b)^10 (12/d)^10 (21/L)^10, where b is the beam width (in.), d is the beam depth (in.), and L is the beam length between the points of zero moment (ft).

(6) The values of F_SG are based on members 5-1/8 inches in width by 12 inches in depth by 21 feet in length. For members with a larger volume, F_SG shall be multiplied by a factor of 0.72. The tabulated F_SG values are for timbers with laminations made from a single piece of lumber across the width or multiple pieces that have been edge bonded. For timber manufactured from multiple piece laminations (across width) that are not edge bonded, value shall be multiplied by 0.4 for members with 5, 7, or 9 laminations or by 0.5 for all other members.

(7) The tabulated E values include true E (also known as "shear-free E"), apparent E, and E for beam stability calculation (NDS 3.3.3.8). For calculating beam deflections, the tabulated E values shall be used. The tabulated E values are determined in addition to bending deflection based on the tabulated E. The axial modulus of elasticity, E, and E, shall be equal to the tabulated E and E values.

(8) The values of F are based on members 12 inches in depth. For depths less than 12 inches, F values shall be permitted to be increased by multiplying by the flat use factor, (12/d)^10, where d is the beam depth in inches. When d is less than 9 inches, use the size adjustment factor for 3 inches.

(9) When containing wane, this combination must be used in dry conditions only. In this case, use-wet factors must not be applied. Because of the wane, this combination is available only for an industrial appearance characteristic. If wane is omitted, these restrictions must not apply. This combination is limited to 9 to 20 laminations in depth, which contains a maximum of 1/8 wane on each side and must be 4 laminations or more in depth.

(10) This combination is limited to nominal 6 inches or less in width.

(11) For members of more than 15 laminations, E = 2.1 x 10^6 psi, E_y = 2.0 x 10^6 psi, and E_y = 1.06 x 10^6 psi. This combination may contain lumber with wane. If lumber with wane is used, the design value for shear parallel to grain, F_SG, shall be multiplied by 0.67 if wane is allowed on both sides. If wane is limited to one side, F_SG shall be multiplied by 0.83. This reduction shall be cumulative with the adjustment in footnote (6).
### Table 2: Allowable Design Values for Glulam Products Manufactured by Mississippi Laminators, Inc. Stressed Primarily in Axial Tension or Compression for Normal Duration of Load\(^{(1)}\)

<table>
<thead>
<tr>
<th>Combination Symbol</th>
<th>Species(^{(2)})</th>
<th>Grade</th>
<th>All Loading</th>
<th>Axially Loaded</th>
<th>Bending about Y-Y Axis</th>
<th>Bending about X-X Axis</th>
<th>Fasteners</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Modulus of Elasticity(^{(3)})</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>(E_x) or (E_y)</td>
<td>(E_x) or (E_y)</td>
<td>(E_x) or (E_y)</td>
<td>(E_x) or (E_y)</td>
<td>(F_{ct}) (psi)</td>
<td>(F_{ct}) (psi)</td>
<td>(F_{ct}) (psi)</td>
</tr>
<tr>
<td>49</td>
<td>SP</td>
<td>N1M16</td>
<td>1.8</td>
<td>1.7</td>
<td>0.90</td>
<td>650</td>
<td>1,350</td>
</tr>
<tr>
<td>50</td>
<td>SP</td>
<td>N1D14</td>
<td>2.0</td>
<td>1.9</td>
<td>1.00</td>
<td>740</td>
<td>1,550</td>
</tr>
</tbody>
</table>

(1) The tabulated allowable design values are for normal duration of loading. For other durations of loading, see applicable building code. The tabulated allowable design values are for dry conditions of use. For wet conditions of use, multiply the tabulated values by the factors shown at the bottom of the table.

(2) SP = Southern pine.

(3) The tabulated \(E\) values include shear-free (true) modulus of elasticity \((E_{x\text{true}}, E_{y\text{true}}, \text{and } E_{\text{axial}})\), apparent modulus of elasticity \((E_{x\text{app}} \text{ and } E_{y\text{app}})\), and 5th percentile modulus of elasticity \((E_{x\text{min}} \text{, } E_{y\text{min}}, \text{and } E_{\text{axial min}})\). For column stability calculation (NDS 3.7.1), \(E_{\text{axial min}}\) shall be used. For calculating the total deflection due to bending, the tabulated \(E_{x\text{app}} \text{ or } E_{y\text{app}}\) values shall be used, or as an alternative, the true (shear-free) bending deflection shall be calculated using the tabulated \(E_{x\text{true}} \text{ or } E_{y\text{true}}\), which shall be added to the calculated shear deflection to determine the total deflection due to bending.

(4) For non-prismatic members, notched members, members subject to impact or cyclic loading, or shear design of bending members at connections (NDS 3.4.3.3), the tabulated \(F_{bx}\) and \(F_{by}\) values shall be multiplied by 0.72.

(5) The tabulated \(F_{by}\) values are for members of 4 or more lams. The tabulated \(F_{by}\) values shall be multiplied by a factor of 0.95 for 3 lams and 0.84 for 2 lams. For members with 5, 7, or 9 lams manufactured from multiple-piece lams with unbonded edge joints, the tabulated \(F_{by}\) values shall be multiplied by a factor of 0.5. This adjustment shall be cumulative with the adjustment specified in Footnote 5.

(6) The values of \(F_{bx}\) are based on members 5-1/8 inches in width by 12 inches in depth by 21 feet in length. For members with a larger volume, \(F_{bx}\) shall be multiplied by a volume factor, \(C_v = (5.125/b)^{1/20} (12/d)^{1/20} (21/L)^{1/20}\), where \(b\) is the beam width (in.), \(d\) is the beam depth (in.), and \(L\) is the beam length between the points of zero moment (ft).

(7) The tabulated \(F_{bx}\) values are for members without special tension lams up to 15 inches in depth. If the member depth is greater than 15 inches without special tension lams, the tabulated \(F_{bx}\) values must be multiplied by a factor of 0.86. If special tension lams are used, the tabulated \(F_{bx}\) values are permitted to be increased by a factor of 1.18 regardless of the member depth provided that the increased \(F_{bx}\) value does not exceed 2,400 psi. This factor shall be cumulative with the volume factor, \(C_v\), specified in Footnote 6.
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