Resolute RSI I-Joists Resolute Engineered Wood Larouche Inc. PR-L340 Revised April 11, 2023

Products: Resolute RSI I-Joists

Resolute Engineered Wood Larouche Inc., 900 chemin du Lac-Hippolyte, Larouche, Quebec G0W 1Z0, Canada (418) 547-2828 www.pfresolu.com

Basis of the product report:

- 2021, 2018, 2015, and 2012 International Building Code (IBC): Sections 104.11 Alternative materials and 2303.1.2 Prefabricated wood I-joists
- 2021, 2018, and 2015 International Residential Code (IRC): Sections 104.11 Alternative materials, and R502.1.2 and R802.1.8 (2021 and 2018 IRC only) Prefabricated wood I-joists
- 2012 IRC: Sections R104.11 Alternative materials and R502.1.4 Prefabricated wood I-joists
- ASTM D5055-16, D5055-13e1, ASTM D5055-13, and D5055-09 recognized in the 2021 IBC and IRC, 2018 IBC and IRC, 2015 IBC and IRC, and 2012 IBC and IRC, respectively
- APA PRI-400 Performance Standard for Residential I-Joists
- 2021, 2015, and 2008 ANSI/AWC Special Design Provisions for Wind and Seismic (SPDWS) recognized in the 2021, 2018 and 2015, and 2012 IBC, respectively
- Intertek LPI 20, LPI 20X1.7 and LPI 32 Test Report, Intertek LPI 20X1.5 Test Report, PFS LPI 23 (a.k.a. LPI 32) Test Report, APA Reports T2005M-21, T2005M-52, T2006M-03, T2006M-07, T2008P-69, T2008P-97, T2008P-111, T2009P-03, T2009P-14, T2009P-60, T2009P-61, T2009P-82, T2010P-36, T2010P-39, T2010P-52B, T2010P-58, T2011P-08, T2013P-35, T2013P-36, T2014P-03, T2014P-29, T2014P-36, T2015L-05B, T2015P-20, T2016P-01, T2016P-27, T2017L-25, and T2017P-32, and other qualification data

2. Product description:

Resolute RSI I-joists are described in Table 1 in accordance with the in-plant manufacturing standard approved by APA.

Design properties:

Tables 2 and 3 lists the design properties for the Resolute RSI I-joists covered by this report. Table 4 shows the allowable lateral shear capacities of Resolute RSI I-joists in diaphragm applications. The allowable spans for Resolute RSI I-joists shall be in accordance with the recommendations provided by the manufacturer (www.pfresolu.com). The allowable spans for Resolute PRI I-joists shall be permitted in accordance with the APA *Performance Rated I-Joists*, Form Z725 (www.apawood.org/resource-library).

4. Product installation:

Resolute RSI I-joists covered by this report shall be installed in accordance with the recommendations provided by the manufacturer (see link above) or the APA *Performance Rated I-Joists*, Form Z725 (see link above) for products qualified as the PRI Series. Permissible web holes and cantilever reinforcements shall be in accordance with the recommendations provided by the manufacturer or with the APA Z725 for products qualified as the PRI Series.

Fire-rated assemblies:

Fire-rated assemblies shall be constructed in accordance with the recommendations provided by the manufacturer or APA *Fire-Rated Systems*, Form W305 (see link above).

Limitations:

- a) Resolute RSI I-joists shall be designed in accordance with the code using the design properties specified in this report.
- b) Resolute RSI I-joists are limited to dry service conditions where the average equilibrium moisture content of solid-sawn lumber is less than 16%.
- c) Resolute RSI I-joists are produced at the Resolute Engineered Wood Larouche Inc. facility in Larouche, Quebec under a quality assurance program audited by APA.
- d) This report is subject to re-examination in one year.

Identification:

Resolute RSI I-joists described in this report are identified by a label bearing the manufacturer's name (Resolute Engineered Wood Larouche Inc.) and/or trademark, the APA assigned plant number (1068), the I-joist series designation and depth, the APA logo, the report number PR-L340, and a means of identifying the date of manufacture.

Table 1. Description of Resolute RSI I-Joists(a)

Joist Joist Depths Series (in.)			Web				
			O (b)	Dime	nsion		Thickness ^(c)
	(In.)	Material	G ^(b)	Depth (in.)	Width (in.)	Material	(in.)
RSI 15	8-7/8 - 16	Proprietary SPF	0.42	1-1/2	2-1/2	OSB	3/8
RSI 25	8-7/8 – 16	Proprietary SPF	0.42	1-1/2	2-1/2	OSB	3/8
RSI 25x4	9-1/2 - 16	MSR SPF	0.42	1-1/2	3-1/2	OSB	3/8
RSI 35	8-7/8 – 16	MSR SPF	0.46	1-1/2	2-1/2	OSB	3/8
RSI 45	8-7/8 - 24	Proprietary SPF	0.46	1-1/2	3-1/2	OSB	3/8 ^(d)
RSI 55	9-1/4 - 24	MSR SPF	0.50	1-1/2	3-1/2	OSB	7/16
RSI 65	11-7/8 - 24	LVL	0.50	1-1/2	3-1/2	OSB	7/16

⁽a) Referenced dimensions are nominal. Tolerances are as specified in the in-plant quality manual.

⁽b) Specific gravity of flanges for use in diaphragm design (see Table 4) based on oven-dry weight and oven-dry volume for lumber flanges or equivalent specific gravity for LVL flanges.

⁽c) 7/16-inch webs shall be permitted to substitute for 3/8-inch webs.

⁽d) 7/16 inch webs for joist depths exceeding 16 inches.

Table 2. Design Properties (Allowable Stress Design) for Resolute RSI I-Joists^(a)

Table 2. Design Properties (Allowable Stress Design) for Resolute RSI I-Joists ^(a)										
Joist Series	Joist Depth (inches, unless	EI ^(b)	M ^(c)	V ^(d)	VLC ^(e)	K ^(f)				
Designation	otherwise noted)	(10 ⁶ lbf-in. ²)	(lbf-ft)	(lbf)	(lbf/ft)	(10 ⁶ lbf-ft/in.)				
	8-7/8	92	2,205	1,055	1,900	0.334				
RSI 15	9-1/4	114	2,315	1,100	1,900	0.347				
	9-1/2 ^(g)	142	2,365	1,130	1,900	0.355				
RSI 15	11-1/4	228	2,915	1,280	1,760	0.414				
	11-7/8 ^(g)	248	3,100	1,335	1,760	0.435				
	14	371	3,720	1,510	1,600	0.508				
	16	514	4,230	1,680	1,200	0.577				
	8-7/8	157	2,580	1,175	1,900	0.337				
	9-1/4	173	2,710	1,225	1,900	0.350				
	240 mm	183	2,795	1,250	1,900	0.356				
	9-1/2 ^(h)	185	2,810	1,260	1,900	0.358				
DOI 05	11-1/4	280	3,410	1,425	1,760	0.417				
RSI 25	300 mm	314	3,735	1,475	1,760	0.436				
	11-7/8 ^(h) 14 ^(h)	318	3,755	1,485	1,760	0.438				
	360 mm	474 488	4,400	1,680	1,600 1,500	0.512 0.518				
	400 mm	629	4,460 4.965	1,700	1,500 1,500	0.573				
	16 ^(h)	652	4.965 5,050	1,845 1,870	1,500 1,500	0.573				
	9-1/2 ^(h)	185	2,810	1,260	1,900	0.358				
RSI 25x4	11-7/8 ^(h)	318	3,755	1,485	1,760	0.438				
	14 ^(h)	474	4,400	1,680	1,600	0.512				
	16 ^(h)	652	5,050	1,870	1,500	0.582				
	8-7/8	203	3,340	1,175	2,200	0.201				
	9-1/4	228	3,510	1,225	2,200	0.208				
	9-1/2 ⁽ⁱ⁾	243	3,620	1,260	2,200	0.213				
RSI 35	11-1/4	359	4,410	1,425	2,200	0.252				
RSI 35	11-7/8 ⁽ⁱ⁾	406	4,690	1,485	2,200	0.267				
	14 ⁽ⁱ⁾	589	5,645	1,680	1,600	0.313				
	16 ⁽ⁱ⁾	791	6,545	1,870	1,500	0.358				
	8-7/8	272	4,955	1,265	2,200	0.385				
	9-1/4	301	5,210	1,310	2,200	0.401				
	240 mm	317	5,340	1,335	2,200	0.410				
	9-1/2	321	5,375	1,340	2,200	0.412				
	11-1/4	480	6,550	1,550	2,200	0.488				
	300 mm	535	6,920	1,615	2,200	0.513				
	11-7/8 ^(j)	547	6,965	1,625	2,200	0.515				
RSI 45	14 ^(j)	802	8,390	1,875	2,000	0.607				
	360 mm	825	8,505	1,895	2,000	0.614				
	400 mm	1,054	9,560	2,085	2,000	0.682				
	16 ^(j)	1,092	9,725	2,115	2,000	0.693				
	18	1,333	11,000	2,555	1,700	0.960				
	20	1,688	12,170	2,795	1,580	1.067				
	22	2,088	13,335	3,030	1,300	1.173				
	24	2,534	14,480	3,270	1,100	1.280				

(Footnotes on Page 4)

	Table 2. D	Design Properties	(Allowable Stress	Design) for	Resolute RSI I-Joists	(a) (Continued)
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Joist Series Designation	Joist Depth (inches, unless otherwise noted)	EI ^(b) (10 ⁶ lbf-in. ²)	M ^(c) (lbf-ft)	V _(d)	VLC ^(e) (lbf/ft)	K ^(f) (10 ⁶ lbf-ft/in.)
	9-1/4	334	6,340	1,715	2,400	0.493
	9-1/2	356	6,540	1,745	2,400	0.507
	11-1/4	529	7,965	1,975	2,400	0.600
	11-7/8	600	8,475	2,055	2,400	0.633
RSI 55	14	874	10,205	2,330	2,200	0.747
KSI 33	16	1,183	11,835	2,585	2,000	0.853
	18	1,540	13,380	2,845	1,700	0.960
	20	1,948	14,810	3,105	1,580	1.067
	22	2,408	16,220	3,360	1,300	1.173
	24	2,919	17,615	3,620	1,100	1.280
	11-7/8	668	10,170	2,055	2,400	0.549
	14	968	12,250	2,330	2,200	0.641
	16	1,301	14,205	2,585	1,900	0.729
RSI 65	18	1,684	16,010	2,845	1,700	0.817
	20	2,115	17,800	3,105	1,580	0.905
	22	2,597	19,575	3,360	1,300	0.993
	24	3,127	21,340	3,620	1,100	1.081

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 lbf = 4.448 N.

Uniform Load:
$$\delta = \frac{5 \omega L^4}{384 \, EI} + \frac{\omega L^2}{12 K} \eqno(1)$$

Center-Point Load:
$$\delta = \frac{PL^3}{48EI} + \frac{2PL}{K}$$
 [2]

where δ = calculated deflection (in.), ω = uniform load (lbf/in.),

P = concentrated load (lbf), L = design span (in.),

EI = bending stiffness of the I-joist (lbf-in.²), and K = coefficient of shear deflection (lbf-ft/in.).

⁽a) The tabulated values are design values for normal duration of load (10 years). All values, except for EI and K, shall be adjusted for other load durations in accordance with the code., and the VLC values shall not be increased for shorter durations.

⁽b) Bending stiffness (EI) of the I-joist.

⁽c) Moment capacity (M) of the I-joist, which shall not be increased by any repetitive member factor.

⁽d) Shear capacity (V) of the I-joist.

⁽e) Uniform vertical load capacity of the I-joist.

⁽f) Coefficient of shear deflection (K). For calculating uniform load and center-point load deflections of the I-joist in a simple-span application, use Eqs. 1 and 2.

⁽⁹⁾ The 9-1/2, and 11-7/8-inch RSI 15 shall be permitted to be designed as PRI-20 I-joists.

⁽h) The 9-1/2, 11-7/8, 14, and 16-inch RSI 25 and RSI-25x4 shall be permitted to be designed as PRI-40 I-joists.

⁽i) The 9-1/2, 11-7/8, 14, and 16-inch RSI 35 shall be permitted to be designed as PRI-60 I-joists.

⁽i) The 11-7/8, 14, and 16-inch RSI 45 are recognized as PRI-80 I-joists.

Table 3. Reaction Capacities (Allowable Stress Design) for Resolute RSI I-Joists^(a,b,c)

Joist Depth			Intermediate F	Reaction(d) (lbf)				Compressive Stress		
Joist Series	(inches, unless	3-1/2 in. Brg. Length		5-1/2 in. B	rg. Length	1-1/2 in. Brg. Length			4 in. Brg. Length	
Designation	otherwise	With Brg. Stiffeners		With Brg. Stiffeners		With Brg. Stiffeners		With Brg. Stiffeners		Perpendicular to
	noted)	No	Yes	No	Yes	No	Yes	No	Yes	Grain (F _{c⊥}), psi
	8-7/8	1,940	2,095	2,165	2,320	870	990	975	1,055	
	9-1/4	1,960	2,115	2,190	2,350	870	1,010	990	1,100	
	9-1/2 ^(f)	1,975	2,135	2,205	2,370	870	1,025	995	1,130	
RSI 15	11-1/4	2,065	2,235	2,300	2,500	870	1,110	1,030	1,280	425
	11-7/8 ^(f)	2,095	2,270	2,335	2,545	870	1,145	1,040	1,335	
	14	2,205	2,395	2,450	2,700	870	1,255	1,080	1,510	
	16	2,310	2,515	2,565	2,855	870	1,355	1,115	1,680	
	8-7/8	2,160	2,330	2,410	2,580	970	1,100	1,085	1,175	
	9-1/4	2,180	2,355	2,435	2,615	970	1,125	1,100	1,225	
	240 mm	2,190	2,370	2,445	2,630	970	1,135	1,105	1,250	
RSI 25	9-1/2 ^(g)	2,195	2,375	2,450	2,635	970	1,140	1,110	1,260	
	11-1/4	2,295	2,485	2,560	2,780	970	1,235	1,145	1,425	
	300 mm	2,325	2,520	2,590	2,825	970	1,270	1,155	1,475	425
	11-7/8 ^(g)	2,330	2,525	2,595	2,830	970	1,275	1,160	1,485	
	14 ^(g)	2,455	2,665	2,725	3,005	970	1,395	1,200	1,680	
	360 mm	2,465	2,675	2,740	3,020	970	1,405	1,205	1,700	
	400 mm	2,555	2,780	2,835	3,150	970	1,495	1,235	1,845	
	16 ^(g)	2,570	2,795	2,850	3,175	970	1,510	1,240	1,870	
	9-1/2 ^(g)	2,195	2,375	2,450	2,635	970	1,140	1,110	1,260	
RSI 25x4	11-7/8 ^(g)	2,330	2,525	2,595	2,830	970	1,275	1,160	1,485	425
1101 2024	14 ^(g)	2,455	2,665	2,725	3,005	970	1,395	1,200	1,680	720
	16 ^(g)	2,570	2,795	2,850	3,175	970	1,510	1,240	1,870	
	8-7/8	2,160	2,330	2,410	2,580	970	1,100	1,085	1,175	
	9-1/4	2,180	2,355	2,435	2,615	970	1,125	1,100	1,225	
	9-1/2 ^(h)	2,195	2,375	2,450	2,635	970	1,140	1,110	1,260	
RSI 35	11-1/4	2,295	2,485	2,560	2,780	970	1,235	1,145	1,425	525
	11-7/8 ^(h)	2,330	2,525	2,595	2,830	970	1,275	1,160	1,485	
	14 ^(h)	2,455	2,665	2,725	3,005	970	1,395	1,200	1,680	
	16 ^(h)	2,570	2,795	2,850	3,175	970	1,510	1,240	1,870	

(Footnotes on Page 7)

Table 3. Reaction Capacities (Allowable Stress Design) for Resolute RSI I-Joists^(a,b,c) (Continued)

	Joist Depth		Intermediate F	Reaction(d) (lbf)						
Joist Series	(inches, unless	3-1/2 in. B	rg. Length	5-1/2 in. B	rg. Length	1-1/2 in. Brg. Length		4 in. Brg. Length		Compressive Stress Perpendicular to
Designation	otherwise	With Brg. Stiffeners		With Brg. Stiffeners		With Brg. Stiffeners		With Brg. Stiffeners		Grain (F _{c⊥}), psi
	noted)	No	Yes	No	Yes	No	Yes	No	Yes	Οιαιτί (1 t±), ροι
	8-7/8	2,870	3,025	2,890	3,105	1,170	1,265	1,240	1,265	
	9-1/4	2,890	3,065	2,920	3,160	1,180	1,310	1,280	1,310	
	240 mm	2,895	3,085	2,935	3,185	1,185	1,335	1,295	1,335	
	9-1/2	2,900	3,095	2,940	3,195	1,185	1,340	1,305	1,340	
	11-1/4	2,995	3,270	3,075	3,430	1,230	1,465	1,515	1,550	
	300 mm	3,020	3,335	3,115	3,505	1,245	1,505	1,585	1,615	
	11-7/8 ⁽ⁱ⁾	3,025	3,340	3,120	3,515	1,245	1,510	1,595	1,625	
RSI 45	14 ⁽ⁱ⁾	3,140	3,565	3,280	3,805	1,300	1,660	1,595	1,875	525
	360 mm	3,150	3,580	3,295	3,830	1,305	1,670	1,595	1,895	
	400 mm	3,230	3,750	3,415	4,045	1,345	1,780	1,595	2,085	
	16 ⁽ⁱ⁾	3,245	3,775	3,435	4,080	1,350	1,800	1,595	2,115	
	18	3,450	4,285	3,850	4,625	1,500 ^(j)	2,305 ^(j)	1,690	2,555	
	20	3,450	4,410	3,850	4,835	1,500 ^(j)	2,450 ^(j)	1,690	2,795	
	22	3,450	4,530	3,850	5,030	1,500 ^(j)	2,595 ^(j)	1,690	3,030	
	24	3,450	4,640	3,850	5,210	1,500 ^(j)	2,705 ^(j)	1,690	3,270	
	9-1/4	3,400	3,680	3,500	3,800	1,330	1,630	1,590	1,715	
	9-1/2	3,400	3,710	3,515	3,840	1,335	1,650	1,600	1,745	
	11-1/4	3,415	3,925	3,605	4,110	1,360	1,775	1,665	1,975	615
	11-7/8	3,420	4,000	3,635	4,210	1,370	1,820	1,690	2,055	
RSI 55	14	3,435	4,260	3,745	4,540	1,385	1,970	1,845	2,330	
KSI 33	16	3,450	4,505	3,850	4,855	1,400	2,110	1,985	2,585	
	18	3,450	4,750	3,850	5,165	1,700 ^(j)	2,490 ^(j)	2,130	2,845	
	20	3,450	4,990	3,850	5,475	1,700 ^(j)	2,675 ^(j)	2,130	3,105	
	22	3,450	5,235	3,850	5,790	1,700 ^(j)	2,865 ^(j)	2,130	3,360	
	24	3,450	5,480	3,850	6,100	1,700 ^(j)	3,055 ^(j)	2,130	3,620	
	11-7/8	3,130	3,860	3,670	4,060	1,145	1,660	1,515	2,055	1
	14	3,130	4,055	3,670	4,300	1,145	1,755	1,535	2,330	
	16	3,130	4,245	3,670	4,525	1,145	1,845	1,555	2,585	
RSI 65	18	3,130	4,435	3,670	4,750	1,315 ^(j)	2,300 ^(j)	1,575	2,845	550
	20	3,130	4,620	3,670	4,975	1,325 ^(j)	2,455 ^(j)	1,595	3,105	
	22	3,130	4,810	3,670	5,200	1,335 ^(j)	2,610 ^(j)	1,615	3,360	
	24	3,130	5,000	3,670	5,430	1,340 ^(j)	2,770 ^(j)	1,635	3,620	

(Footnotes on Page 7)

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 lbf = 4.448 N, 1 psi =6.895 kPa.

- (a) Reaction capacity shall be limited by the tabulated I-joist reaction capacity, flange bearing capacity, or the bearing capacity of the support material, whichever is less. The flange bearing capacity is based on the allowable compressive stress perpendicular to grain of the I-joist flange, the net flange width, and the bearing length, and may be further limited by the bearing capacity of the support material. To calculate the net flange width, subtract 0.25 inch from the flange width (see Table 1) of the RSI 15, RSI 25x4, RSI 35, RSI 45, and RSI 55 I-joists, or subtract 0.10 inch from the flange width (see Table 1) of the RSI 65 I-joists.
- (b) Reaction capacity is for normal duration of load and shall be adjusted for other load durations provided that the adjusted reaction design value is not greater than the flange bearing capacity or the bearing capacity of the support material. Flange bearing capacity and the bearing capacity of any wood support shall not be adjusted for load duration.
- (c) Reaction capacity and flange bearing capacity shall be permitted to be increased over that tabulated for the minimum bearing length. Linear interpolation of the reaction capacity between the minimum and maximum bearing length is permitted. Bearing lengths longer than the maximum do not further increase the reaction capacity. Flange bearing capacity and that of a wood support will increase with additional bearing length.
- (d) For depths of 9-1/2 inches and greater, the intermediate reaction with a minimum bearing length of 3 inches shall be permitted to be determined based on the intermediate reaction values with a bearing length of 3-1/2 inches and 5-1/2 inches.
- (e) The minimum bearing length for end reactions is 1-1/2 inches, unless otherwise noted.
- The 9-1/2 and 11-7/8-inch RSI 15 shall be permitted to be designed as PRI-20 I-joists.
- (9) The 9-1/2, 11-7/8, 14, and 16-inch RSI 25 and RSI 25x4 shall be permitted to be designed as PRI-40 I-joists.
- (h) The 9-1/2, 11-7/8, 14, and 16-inch RSI 35 shall be permitted to be designed as PRI-60 I-joists.
- (i) The 11-7/8, 14, and 16-inch RSI 45 are recognized as PRI-80 I-joists.
- (i) Minimum bearing length is 2-1/2 inches.

Table 4. Allowable Shear (Pounds Per Foot) for Horizontal Wood Structural Panel Diaphragms Framed With Resolute RSI I-Joists for Wind^(a) or Seismic Loading^(b,c)

VVIIId	ocisinic Lo	J		Blo	cked Diaphrag	ms	Unblocked Diaphragms	
Panel Grade	Common Nail Size	Minimum Nominal Panel	Minimum Nominal Width of Framing Members at Adjoining	boundaries panel edges	icing (in.) at dia (all cases), at parallel to load Ill panel edges 6) ^(f,g)	continuous d (Cases 3 &	Nails Spaced 6 in. max. at supported edges ^(f,g)	
	Ivali Size	Thickness	Panel Edges and Boundaries ^(e) (in.)	6	4 ^(h)	2-1/2 ⁽ⁱ⁾	Case 1 (No	All other
		(in.)			g (in.) at other <mark>բ</mark> ases 1, 2, 3, &		unblocked edges or continuous	configurations (Cases 2, 3, 4, 5 &6)
				6	6	4	joints parallel to load	
	6d ^(d)	5/16	3	210	280	420	185	140
Structural 1 Grades	8d	3/8		300	400	600	265	200
Grades	10d	15/32		360	480	720	320	240
	6d ^(d)	5/16		190	250	380	170	125
Objection of single		3/8		210	280	420	185	140
Sheathing, single floor and other		3/8		270	360	540	240	180
grades covered	8d	7/16		285	380	570	255	190
in DOC PS 1 and		15/32		300	400	600	265	200
PS 2	104	15/32		325	430	650	290	215
	10d	19/32		360	480	720	320	240

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 lbf = 4.448 N, 1 lbf/ft = 0.0146 N/mm.

(Footnotes on the following pages)

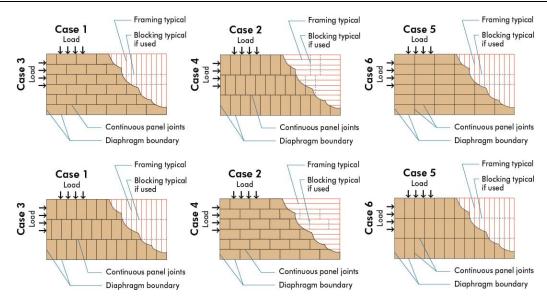


Figure 1. Diaphragm configurations

- (a) For wind load applications, the values in the table above shall be permitted to be multiplied by 1.4.
- (b) For shear loads of normal or permanent load duration as defined by the NDS, the values in the table above shall be multiplied by 0.63 or 0.56, respectively.
- (c) The tabulated allowable shear capacities are for I-joist series with flanges having a specific gravity (G) of 0.50 or higher (see Table 1). For G < 0.50 the allowable shear capacities shall be reduced by multiplying the allowable shear capacities by the Specific Gravity Adjustment Factor = [1-(0.5-G)]. The Specific Gravity Adjustment Factor shall not be greater than 1.
- (d) 8d common nails minimum are recommended for roofs due to negative pressures of high winds.
- (e) The minimum nominal width of framing members not located at boundaries or adjoining panel edges shall be 2 inches.
- (f) Space nails maximum 12 inches o.c. along intermediate framing members (6 inches o.c. when supports are spaced 48 inches o.c. or greater).
- (9) Fasteners shall be located 3/8 inch minimum from panel edges (see Figures 2, 3 and 4).
- h) Adjacent nails within a row must be staggered ½ inch when nail spacing is 4 inches or less (see Figure 3).
- (i) Adjacent nails within a row must be staggered ½ inch at adjoining panel edges when nail spacing is 2-½ inches o.c. (see Figure 4).

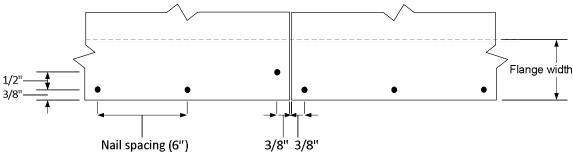


Figure 2. Boundary nails for nail spacing of 6 inches o.c. (not to scale)

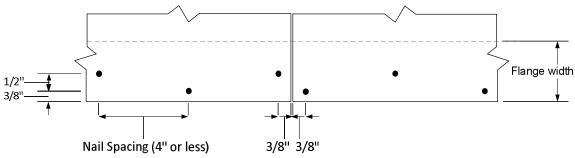


Figure 3. Staggered nails when the nail spacing is 4 inches o.c. or less at diaphragm boundaries (not to scale)

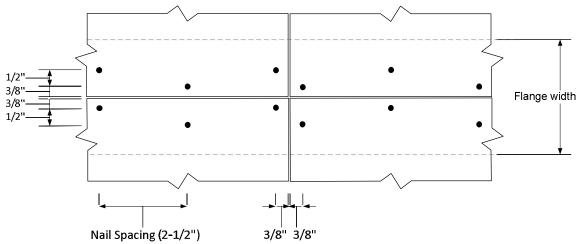


Figure 4. Staggered nails when the nail spacing is 2-1/2 inches o.c. at adjoining panel edges (not to scale)

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